

## Window Installation Analysis

**QUAKER WINDOWS & DOORS**  
**M600 Awning-Fixed-Awning**  
**M600 Awn-Fix & Fix-Awn**

Report Q3930.01-122-34

Rendered to:

QUAKER WINDOWS & DOORS  
P.O. Box 128  
504 Highway 63 South  
Freeburg, Missouri 65035

Prepared by:

Tanya A. Dolby, P.E.  
Adam R. Kunkel

Architectural Testing, Inc.  
130 Derry Court  
York, Pennsylvania 17406  
(717) 764-7700  
Florida COA: 29274


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Tanya A. Dolby, P.E.  
Manager, Engineering Services

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Adam R. Kunkel  
Project Engineer

	<b>PROJECT:</b> Installation Calcs – M600 FIXED/AWNING/FIXED	<b>BY:</b> TAD <b>DATE:</b> 09/29/23
	<b>PROJECT NO.:</b> Q3930.01-122-34	<b>CKD:</b> ARK <b>SHEET:</b> 2 OF 25

**Scope**

Architectural Testing, Inc., an Intertek company, was contracted by Quaker Windows & Doors to evaluate alternate installation methods for their M600 Fixed over Awning with Roto-Operator over Fixed windows. The evaluation is based on physical testing and product certifications.

Reference standards utilized in this project include:

*Florida Building Code, Building, 8<sup>th</sup> Edition (2023).* International Code Council, 2023.

*ANSI/AWC NDS-2018 National Design Specification (NDS) for Wood Construction with 2015 Supplement.* American Wood Council, 2018.


*ADM1-2020 Aluminum Design Manual.* The Aluminum Association, Inc., 2020.

*AISI S100-16(2020)w/S2-20 North American Specification for the Design of Cold-Formed Steel Structural Members, 2016 Edition(Reaffirmed 2020).* American Iron and Steel Institute, 2020.

*ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners.* ICC Evaluation Service. 07/2023.

*NOA 21-0628.20 Hilti Kwik-Con+ Concrete and Masonry Screw Anchor.* Miami-Dade County Product Control Section. 08/19/2021.


The anchorage analyses presented herein do not address the water resistance, water penetration or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding incurred loads.

	PROJECT: Installation Calcs – M600 FIXED/AWNING/FIXED	BY: TAD    DATE: 09/29/23
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**Certification of Independence**

In accordance with Rule 61G20-3 Florida Administrative Code, Architectural Testing, Inc. hereby certifies the following:

- Architectural Testing does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
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**Analyses**

**Summary of Test Results**

The following table summarizes the M600 Fixed over Awning with Roto-Operator over Fixed Window products and their corresponding performance levels which have been established by testing or product certification.

**Table 1** Summary of Test Results

Series/Model	Test Report Number	Product Certification	Size (W x H)	Performance
M600 Fixed over Awning with Roto-Operator over Fixed Window	L5050.01-801-44 (Revision 1, 05/18/22)	NI015457-R2	60" x 144"	+/- 70 psf

L5050.01-801-44 testing was conducted by the Architectural Testing laboratory in Plano, Texas (Florida Department of Business & Professional Regulation Test Lab No. TST1910, IAS Accredited Laboratory TL-331).

**As-Tested Installation Analysis**

For air/water/structural testing, the test specimen was secured to a 2x Spruce-Pine-Fir wood buck with nailing fins using #8 x 1-5/8" wood screws. The as-tested installation method is evaluated on page 8 and the established design capacities are summarized in Table 2.

**Table 2** As-Tested Anchorage Design Capacity

Test	Connection	Capacity
M600 Fixed over Awning with Roto-Operator over Fixed Window Air/Water/Structural Test	Nailing Fin with #8 x 1-5/8" Wood Screws Placed 3" from each corner and 15" on center	99 lb

The capacities presented in Table 2 will be used to prove acceptable alternate anchors and substrates for the windows.

### Alternate Anchorages

Calculations on page 9 through page 21 determine the design capacity of alternate anchorages for the windows. The alternate anchorage capacities are summarized in Table 3.

**Table 3** Alternate Anchorage Capacities

Installation	Connection	Capacity	Comments
Nailing Fin to Wood	#10 Wood Screw connecting Nailing Fin to Wood Blocking	121 lb	<ol style="list-style-type: none"> <li>Limited by pull-over</li> <li>1-1/2" min penetration</li> <li>Min Spruce-Pine-Fir wood blocking, G = 0.42</li> </ol>
Nailing Fin to Steel	#10-16 TEKS Screw connecting Nailing Fin to Light Gauge Steel Framing	116 lb	<ol style="list-style-type: none"> <li>Limited by pull-out</li> <li>Full penetration +3 threads</li> <li>Min 18 gauge 33 KSI steel</li> </ol>
Receptor to Wood	#12 Wood Screw connecting Receptor to Wood Blocking	193 lb	<ol style="list-style-type: none"> <li>Limited by shear</li> <li>1-1/2" min penetration</li> <li>1/4" max shim space</li> <li>Min Spruce-Pine-Fir wood blocking, G = 0.42</li> </ol>
Receptor to Steel	#12-14 TEKS Screw connecting Receptor to Light Gauge Steel Framing	209 lb	<ol style="list-style-type: none"> <li>Limited by bending</li> <li>Full penetration +3 threads</li> <li>1/4" max shim space</li> <li>Min 18 gauge 33 KSI steel</li> </ol>
Receptor to Concrete	1/4" Hilti Kwik-Con+ Anchor connecting Receptor to Concrete	183 lb	<ol style="list-style-type: none"> <li>Limited by bending</li> <li>1" min embedment</li> <li>2-1/2" min edge distance</li> <li>2" min spacing</li> <li>1/4" max shim space</li> <li>Min <math>f'_c = 3,000</math> psi concrete</li> </ol>
Receptor to CMU	1/4" Hilti Kwik-Con+ Anchor connecting Receptor to CMU	183 lb	<ol style="list-style-type: none"> <li>Limited by bending</li> <li>1" min embedment</li> <li>2-1/2" min edge distance</li> <li>3" min spacing</li> <li>1/4" max shim space</li> <li>Min ASTM C90 masonry</li> </ol>

**Table 3** Alternate Anchorage Capacities (continued)

Installation	Connection	Capacity	Comments
Trim Clip to Wood	#12 Wood Screw connecting Trim Clip to Wood Blocking	100 lb	<ol style="list-style-type: none"> <li>Limited by connection to window frame</li> <li>1-1/2" min penetration</li> <li>Min Spruce-Pine-Fir wood blocking, G = 0.42</li> </ol>
Trim Clip to Steel	#12-14 TEKS Screw connecting Trim Clip to Light Gauge Steel Framing	100 lb	<ol style="list-style-type: none"> <li>Limited by connection to window frame</li> <li>Full penetration +3 threads</li> <li>Min 18 gauge 33 KSI steel</li> </ol>
Trim Clip to Concrete	1/4" Hilti Kwik-Con+ Anchor connecting Trim Clip to Concrete	100 lb	<ol style="list-style-type: none"> <li>Limited by connection to window frame</li> <li>1" min embedment</li> <li>2-1/2" min edge distance</li> <li>2" min spacing</li> <li>Min <math>f'_c = 3,000</math> psi concrete</li> </ol>
Trim Clip to CMU	1/4" Hilti Kwik-Con+ Anchor connecting Trim Clip to CMU	100 lb	<ol style="list-style-type: none"> <li>Limited by connection to window frame</li> <li>1" min embedment</li> <li>2-1/2" min edge distance</li> <li>3" min spacing</li> <li>Min ASTM C90 masonry</li> </ol>

**Note:** A #10-16 TEKS screw is used to connect the trim clip to window frame. The capacity of this connection is 100 lb as shown on page 21. This connection governs the capacity of all trim clip installation methods.

## **Anchorage Requirements**

Alternate anchorage conditions have anchorage capacities which are comparable to or exceed the as-tested anchorage capacities. The as tested spacings for each anchoring system will apply to alternate substrates.

## **Reference Drawings**

The reference drawings are the basis of the analysis presented herein and may not reflect the requirements established by this analysis.

- *M600 Awn-Fix & Fix-Awn Installation Instructions*. Sheets 1 - 7. Quaker Windows and Doors. 10/05/23. (7 pages)
- *M600 Fixed-Awning-Fixed Installation Instructions*. Sheets 1 - 7. Quaker Windows and Doors. 10/05/23. (7 pages)

**As-Tested Installation – Nailing Fin to Wood Blocking**

#8 x 1-5/8" Wood Screw (Non-Countersunk)  
 1-1/2" Minimum Penetration  
 1/16" thick 6063-T6 Aluminum Nailing Fin  
 G = 0.42 Minimum Spruce-Pine-Fir 2x Wood Blocking

Withdrawal of #8 Wood Screw

$$W' = 2,850(G^2)(D)(C_d)(C_m^2)(C_t)(C_{eg})(C_{tn})(L)$$

$$W' = 2,850(0.42)^2(0.164")(1.6)(1.0)^2(1.0)(1.0)(1.0)(1.5")$$

$$W' = 198 \text{ lb}$$

Pull-Over of #8 Wood Screw

$$P_{nov} = C_{pov}t_1F_{tu1}(D_{ws}-D_h)/3.0$$

$$P_{nov} = 1.0(0.0625")(30,000 \text{ psi})(0.322" - 0.164")/3.0$$

$$P_{nov} = 99 \text{ lb}$$

**Capacity of Connection is 99 lb**

**As-Tested Installation – Trim Clip to Wood Blocking**

#12 Wood Screw (Non-Countersunk)  
 1-1/2" Minimum Penetration  
 1/16" thick 6063-T6 Aluminum Trim Clip  
 G = 0.42 Minimum Spruce-Pine-Fir 2x Wood Blocking (Qualifies Southern Yellow Pine)  
 1/4" Max Shim Space

Allowable Shear of #12 Wood Screw

$$Z' = 193 \text{ lb}$$

Bearing of #12 Screw

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.216")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 270 \text{ lb}$$

Bending of #12 Wood Screw

$$S = \pi d^3/32 = \pi(0.216")^3/32 = 0.001 \text{ in}^3$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(80,000 \text{ psi}) = 62,400 \text{ psi (1.3 factor for weak axis bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.001 \text{ in}^3)(62,400 \text{ psi})/0.25" = 316 \text{ lb}$$

**Capacity of Connection is 100 lb (See Page 21)**



**Alternate Installation – Nailing Fin to Wood Blocking**

#10 Wood Screw (Non-Countersunk)

1-1/2" Minimum Penetration

1/16" thick 6063-T6 Aluminum Nailing Fin

G = 0.42 Minimum Spruce-Pine-Fir 2x Wood Blocking (Qualifies Southern Yellow Pine)

Withdrawal of #10 Wood Screw

$$W' = 2,850(G^2)(D)(C_d)(C_m^2)(C_t)(C_{eg})(C_{tn})(L)$$

$$W' = 2,850(0.42)^2(0.190")(1.6)(1.0)^2(1.0)(1.0)(1.0)(1.5")$$

$$W' = 229 \text{ lb}$$

Pull-Over of #10 Wood Screw

$$P_{nov} = C_{pov}t_1F_{tu1}(D_{ws}-D_h)/3.0$$

$$P_{nov} = 1.0(0.0625")(30,000 \text{ psi})(0.385" - 0.190")/3.0$$

$$P_{nov} = 121 \text{ lb}$$

**Capacity of Connection is 121 lb**

**Alternate Installation – Nailing Fin to Steel Stud**

#10-16 TEKS Screw

Full Penetration +3 Threads

1/16" thick 6063-T6 Aluminum Nailing Fin

Minimum 18 Gauge 33 KSI Steel Stud (Qualifies thicker and stronger steel)

Allowable Tension of #10-16 TEKS Screw

$$V_a = 885 \text{ lb} \quad (\text{ESR-1976})$$

Pull-Over of #10-16 TEKS Screw in Nail Fin

$$P_{nov} = C_{pov} t_1 F_{tu1} (D_{ws} - D_h) / 3.0$$

$$P_{nov} = 1.0(0.0625")(30,000 \text{ psi})(0.400" - 0.190") / 3.0$$

$$P_{nov} = 131 \text{ lb}$$

Pull-Out of #10-16 TEKS Screw in Steel Stud

$$P_{not} = 0.85 t_c d F_{u2} / 3.0$$

$$P_{not} = 0.85(0.0478")(0.190")(45,000 \text{ psi}) / 3.0$$

$$P_{not} = 116 \text{ lb}$$

**Capacity of Connection is 116 lb**

**Alternate Installation – Receptor to Wood Blocking**

#12 Wood Screw (Non-Countersunk)

1-1/2" Minimum Penetration

1/16" thick 6063-T6 Aluminum Receptor

G = 0.42 Minimum Spruce-Pine-Fir 2x Wood Blocking (Qualifies Southern Yellow Pine)

1/4" Maximum Shim Space

11/16" Maximum Bending Space

**Allowable Shear of #12 Wood Screw**

$$Z' = 193 \text{ lb}$$

See Following Page

**Bearing of #12 Screw**

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.216")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 270 \text{ lb}$$

**Bending of #12 Wood Screw**

$$S = \pi d^3/32 = \pi(0.216")^3/32 = 0.001 \text{ in}^3$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(80,000 \text{ psi}) = 62,400 \text{ psi (1.3 factor for weak axis bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.001 \text{ in}^3)(62,400 \text{ psi})/0.375" = 329 \text{ lb}$$

**Capacity of Connection is 193 lb**

## Alternate Installation – Receptor to Wood Blocking (continued)

### Lateral Design Strength of Wood Connections

ANSI / AF&amp;PA NDS-2018

#### Data

**Fastener**

Fastener	=	#12 Wood Screw
Shank Dia	=	0.216 in.
Root Dia.	=	0.171 in.
F <sub>yb</sub>	=	80,000 psi
Fastener length	=	2.000 in.

**Main Member**

Material	=	SPF
G	=	0.42
θ	=	90 ≤ (Angle of load to grain)
F <sub>e</sub>	=	3,350 psi (Table 12.3.2)
Thickness	=	1.500 in.

**Side Member**

Material	=	6063 T6 Aluminum
G	=	N/A
θ	=	0 ≤ (Angle of load to grain)
F <sub>es</sub>	=	37,500 psi
Thickness	=	0.063 in.

#### Calculations

**Lateral Bearing Factors**

D	=	0.171 in.
ℓ <sub>m</sub>	=	1.500 in.
K <sub>θ</sub>	=	1.25 (Table 12.3.1B)
K <sub>D</sub>	=	2.21 (Table 12.3.1B)
R <sub>e</sub>	=	0.089 (Table 12.3.1A)
R <sub>t</sub>	=	24.00 (Table 12.3.1A)
k <sub>1</sub>	=	0.8662 (Table 12.3.1A)
k <sub>2</sub>	=	0.5564 (Table 12.3.1A)
k <sub>3</sub>	=	15.53 (Table 12.3.1A)

Yield Mode	R <sub>d</sub>
I <sub>m</sub> , I <sub>s</sub>	2.21 (Table 12.3.1B)
II	2.21 (Table 12.3.1B)
III <sub>m</sub> , III <sub>s</sub> , IV	2.21 (Table 12.3.1B)

**Lateral Design Values, Z**

Mode I <sub>m</sub>	=	389 lbf (Eq 12.3-1)
Mode I <sub>s</sub>	=	181 lbf (Eq 12.3-2)
Mode II	=	157 lbf (Eq 12.3-3)
Mode III <sub>m</sub>	=	184 lbf (Eq 12.3-4)
Mode III <sub>s</sub>	=	120 lbf ≤ Min Value (Eq 12.3-5)
Mode IV	=	169 lbf (Eq 12.3-6)
C <sub>D</sub>	=	1.6 (B.2)

**Wet Service Factor**

Fabrication/In-Service	=	Dry/Dry
C <sub>M</sub>	=	1.0 (Table 11.3.3)
In service temperature	=	T ≤ 100°F
C <sub>t</sub>	=	1.0 (Table 11.3.4)
C <sub>g</sub>	=	1.0 (11.3.6)
C <sub>Δ</sub>	=	1.0 (12.5.1)
Installed in end grain?	=	No
C <sub>eg</sub>	=	1.00 (12.5.2)
Part of a diaphragm?	=	No
C <sub>di</sub>	=	1.0 (12.5.3)
Toe-nailed?	=	No
C <sub>tn</sub>	=	1.00 (12.5.4)
Z'	=	<b>193 lbf</b> (Table 11.3.1)

**Alternate Installation – Receptor to Steel Stud**

#12-14 TEKS Screw

Full Penetration +3 Threads

1/16" thick 6063-T6 Aluminum Receptor

Minimum 18 Gauge 33 KSI Steel Stud (Qualifies thicker and stronger steel)

1/4" Maximum Shim Space

11/16" Maximum Bending Space

Allowable Shear of #12-14 TEKS Screw

$$V_a = 724 \text{ lb} \quad (\text{ESR-1976})$$

Bearing of #12-14 TEKS Screw on Receptor

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.216")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 270 \text{ lb}$$

Bearing of #12-14 TEKS Screw on Steel Stud

$$V_a = 2.7DtF_u/n_u$$

$$V_a = 2.7(0.216")(0.0478")(45,000 \text{ psi})/3.0$$

$$V_a = 418 \text{ lb}$$

Tilting of #12-14 TEKS Screw in Steel

$$V_a = 4.2(t_2^3D)^{1/2}F_{tu}/n_s$$

$$V_a = 4.2(0.0478^3 \times 0.216)^{1/2}(45,000 \text{ psi})/3.0$$

$$V_a = 306 \text{ lb}$$

Bending of #12-14 TEKS Screw

$$S = \pi d^3/32 = \pi(0.216")^3/32 = 0.001 \text{ in}^3$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(92,000 \text{ psi}) = 71,760 \text{ psi} \text{ (1.3 factor for weak axis bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.001 \text{ in}^3)(71,760 \text{ psi})/0.6875" = 209 \text{ lb}$$

**Capacity of Connection is 209 lb**

**Alternate Installation – Receptor to Concrete**

1/4" Hilti Kwik-Con+ Anchor

1" Minimum Embedment

2-1/2" Minimum Edge Distance

2" Minimum Spacing

1/16" thick 6063-T6 Aluminum Receptor

Minimum  $f'_c = 3,000$  psi Concrete

1/4" Maximum Shim Space

11/16" Maximum Bending Space

Allowable Shear of 1/4" Hilti Kwik-Con+ Anchor

$$P_{ss}/\Omega = 379 \text{ lb} \quad (\text{NOA-No. 21-0628.20})$$

Bearing of 1/4" Hilti Kwik-Con+ in Receptor

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.25")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 313 \text{ lb}$$

Bending of 1/4" Hilti Kwik-Con+

$$S = \pi d^3/32 = \pi(0.190")^3/32 = 0.000673 \text{ in}^3$$

$$F_y = 120,000 \text{ psi per Miami Dade NOA 20-0427.13}$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(120,000 \text{ psi}) = 93,600 \text{ psi (1.3 factor for rod bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.000673 \text{ in}^3)(93,600 \text{ psi})/0.6875" = 183 \text{ lb}$$

**Capacity of Connection is 183 lb**

**Alternate Installation – Receptor to CMU**

1/4" Hilti Kwik-Con+ Anchor

1" Minimum Embedment

2-1/2" Minimum Edge Distance

3" Minimum Spacing

1/16" thick 6063-T6 Aluminum Receptor

Minimum f'm = 1,500 psi ASTM C90 Concrete Masonry

1/4" Maximum Shim Space

11/16" Maximum Bending Space

Allowable Shear of 1/4" Hilti Kwik-Con+ Anchor

$$P_{ss}/\Omega = 251 \text{ lb} \quad (\text{NOA-No. 21-0628.20})$$

Bearing of 1/4" Hilti Kwik-Con+ in Receptor

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.25")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 313 \text{ lb}$$

Bending of 1/4" Hilti Kwik-Con+

$$S = \pi d^3/32 = \pi(0.190")^3/32 = 0.000673 \text{ in}^3$$

$$F_y = 120,000 \text{ psi per Miami Dade NOA 20-0427.13}$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(120,000 \text{ psi}) = 93,600 \text{ psi (1.3 factor for rod bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.000673 \text{ in}^3)(93,600 \text{ psi})/0.6875" = 183 \text{ lb}$$

**Capacity of Connection is 183 lb**

**Alternate Installation – Trim Clip to Wood Blocking**

#12 Wood Screw (Non-Countersunk)

1-1/2" Minimum Penetration

1/16" thick 6063-T6 Aluminum Trim Clip

G = 0.42 Minimum Spruce-Pine-Fir 2x Wood Blocking (Qualifies Southern Yellow Pine)

1/4" Max Shim Space

Allowable Shear of #12 Wood Screw

$$Z' = 193 \text{ lb}$$

See Following Page

Bearing of #12 Screw

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.216")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 270 \text{ lb}$$

Bending of #12 Wood Screw

$$S = \pi d^3/32 = \pi(0.216")^3/32 = 0.001 \text{ in}^3$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(80,000 \text{ psi}) = 62,400 \text{ psi (1.3 factor for weak axis bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.001 \text{ in}^3)(62,400 \text{ psi})/0.25" = 316 \text{ lb}$$

**Capacity of Connection is 193 lb**



## Alternate Installation – Trim Clip to Wood Blocking (continued)

### Lateral Design Strength of Wood Connections

ANSI / AF&amp;PA NDS-2018

#### Data

**Fastener**

Fastener =	#12 Wood Screw
Shank Dia =	0.216 in.
Root Dia. =	0.171 in.
F <sub>yb</sub> =	80,000 psi
Fastener length =	2.000 in.

**Main Member**

Material =	SPF
G =	0.42
θ =	90 ≤ (Angle of load to grain)
F <sub>e</sub> =	3,350 psi (Table 12.3.2)
Thickness =	1.500 in.

**Side Member**

Material =	6063 T6 Aluminum
G =	N/A
θ =	0 ≤ (Angle of load to grain)
F <sub>es</sub> =	37,500 psi
Thickness =	0.063 in.

#### Calculations

**Lateral Bearing Factors**

D =	0.171 in.
ℓ <sub>m</sub> =	1.500 in.
K <sub>θ</sub> =	1.25 (Table 12.3.1B)
K <sub>D</sub> =	2.21 (Table 12.3.1B)
R <sub>e</sub> =	0.089 (Table 12.3.1A)
R <sub>t</sub> =	24.00 (Table 12.3.1A)
k <sub>1</sub> =	0.8662 (Table 12.3.1A)
k <sub>2</sub> =	0.5564 (Table 12.3.1A)
k <sub>3</sub> =	15.53 (Table 12.3.1A)

Yield Mode	R <sub>d</sub>
I <sub>m</sub> , I <sub>s</sub>	2.21 (Table 12.3.1B)
II	2.21 (Table 12.3.1B)
III <sub>m</sub> , III <sub>s</sub> , IV	2.21 (Table 12.3.1B)

**Lateral Design Values, Z**

Mode I <sub>m</sub> =	389 lbf	(Eq 12.3-1)
Mode I <sub>s</sub> =	181 lbf	(Eq 12.3-2)
Mode II =	157 lbf	(Eq 12.3-3)
Mode III <sub>m</sub> =	184 lbf	(Eq 12.3-4)
Mode III <sub>s</sub> =	120 lbf	==== Min Value (Eq 12.3-5)
Mode IV =	169 lbf	(Eq 12.3-6)
C <sub>D</sub> =	1.6	(B.2)

**Wet Service Factor**

Fabrication/In-Service	Dry/Dry
C <sub>M</sub> =	1.0 (Table 11.3.3)
In service temperature	T ≤ 100°F
C <sub>t</sub> =	1.0 (Table 11.3.4)
C <sub>g</sub> =	1.0 (11.3.6)
C <sub>Δ</sub> =	1.0 (12.5.1)
Installed in end grain?	No
C <sub>eg</sub> =	1.00 (12.5.2)
Part of a diaphragm?	No
C <sub>di</sub> =	1.0 (12.5.3)
Toe-nailed?	No
C <sub>tn</sub> =	1.00 (12.5.4)
<b>Z' =</b>	<b>193 lbf</b> (Table 11.3.1)

**Alternate Installation – Trim Clip to Steel Stud**

#12-14 TEKS Screw

Full Penetration +3 Threads

1/16" thick 6063-T6 Aluminum Trim Clip

Minimum 18 Gauge 33 KSI Steel Stud (Qualifies thicker and stronger steel)

1/4" Maximum Shim Space

No Bending

Allowable Shear of #12-14 TEKS Screw

$$V_a = 724 \text{ lb} \quad (\text{ESR-1976})$$

Bearing of #12-14 TEKS Screw on Trim Clip

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.216")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 270 \text{ lb}$$

Bearing of #12-14 TEKS Screw on Steel Stud

$$V_a = 2.7DtF_u/n_u$$

$$V_a = 2.7(0.216")(0.0478")(45,000 \text{ psi})/3.0$$

$$V_a = 418 \text{ lb}$$

Tilting of #12-14 TEKS Screw in Steel

$$V_a = 4.2(t_2^3D)^{1/2}F_{tu}/n_s$$

$$V_a = 4.2(0.0478^3 \times 0.216)^{1/2}(45,000 \text{ psi})/3.0$$

$$V_a = 306 \text{ lb}$$

**Capacity of Connection is 270 lb**

**Alternate Installation – Trim Clip to Concrete**

1/4" Hilti Kwik-Con+ Anchor

1" Minimum Embedment

2-1/2" Minimum Edge Distance

2" Minimum Spacing

1/16" thick 6063-T6 Aluminum Trim Clip

Minimum  $f'_c = 3,000$  psi Concrete

1/4" Maximum Shim Space

No Bending

Allowable Shear of 1/4" Hilti Kwik-Con+ Anchor

$$P_{ss}/\Omega = 379 \text{ lb} \quad (\text{NOA-No. 21-0628.20})$$

Bearing of 1/4" Hilti Kwik-Con+ in Trim Clip

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.25")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 313 \text{ lb}$$

**Capacity of Connection is 313 lb**

**Alternate Installation – Trim Clip to CMU**

1/4" Hilti Kwik-Con+ Anchor

1" Minimum Embedment

2-1/2" Minimum Edge Distance

3" Minimum Spacing

1/16" thick 6063-T6 Aluminum Trim Clip

Minimum f'm = 1,500 psi ASTM C90 Concrete Masonry

1/4" Maximum Shim Space

No Bending

**Allowable Shear of 1/4" Hilti Kwik-Con+ Anchor**

$$P_{ss}/\Omega = 251 \text{ lb} \quad (\text{NOA-No. 21-0628.20})$$

**Bearing of 1/4" Hilti Kwik-Con+ in Receptor**

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.25")(0.0625")(30,000 \text{ psi})/3.0$$

$$V_a = 313 \text{ lb}$$

**Capacity of Connection is 251 lb**

**Alternate Installation – Trim Clip to Window Frame**

#10-16 TEKS Screw

Full Penetration +3 Threads

1/16" thick 6063-T6 Aluminum Trim Clip

1/16" thick 6063-T6 Aluminum Window Frame

Allowable Tension of #10-16 TEKS Screw

$$V_a = 885 \text{ lb} \quad (\text{ESR-1976})$$

Pull-Over of #10-16 TEKS Screw in Trim Clip

$$P_{nov} = C_{pov} t_1 F_{tu1} (D_{ws} - D_h) / 3.0$$

$$P_{nov} = 1.0(0.0625")(30,000 \text{ psi})(0.400" - 0.190") / 3.0$$

$$P_{nov} = 131 \text{ lb}$$

Pull-Out of #10-16 TEKS Screw in Window Frame

$$P_{not} = K_s D L_e F_{ty2} / 3.0$$

$$P_{not} = 1.01(0.190")(0.0625")(25,000 \text{ psi}) / 3.0$$

$$P_{not} = 100 \text{ lb}$$

**Capacity of Connection is 100 lb**

**Tested capacity of M2078 Interior Snap Trim (1/16" thick) determined as shown below on Q3412.01-550-44-R0. Qualified for use at the design capacity of 100 lb.**

Trim&Clip	70.0 psf					
Anchor Spacing	8.0 inch					
Window Mark	Width, w (inch)	Height, h (inch)	w/h	gamma	R (lb/inch)	Anchor Capacity for Specified Spacing (lb)
Q3412.01-550-44-R0	60.00	99.00	1.65	0.494	14.40	115



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**Actual Tested Anchorage Capacity**

<b>Nailfin</b>						
<b>Design Pressure</b>	70.0 psf					
<b>Anchor Spacing</b>	15.0 inch	Head/Sill				
<b>Anchor Spacing</b>	15.0 inch	Jambs				
	<b>Width, w</b>	<b>Height, h</b>			<b>R</b>	<b>Anchor Capacity for</b>
<b>Window Mark</b>	<b>(inch)</b>	<b>(inch)</b>	<b>w/h</b>	<b>gamma</b>	<b>(lb/inch)</b>	<b>Specified Spacing</b>
						<b>(lb)</b>
L5050.01-801-44-R1	60.00	144.00	2.40	0.505	14.74	221
L5050.01-801-44-R1	60.00	60.00	1.00	0.420	12.25	184

**GLASS ANALYSIS**

**Glazing Information**

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Supported Edges: Four sides simply supported  
 Shape: Rectangular  
 Lite Width: 57.1 in.  
 Lite Height: 47.7 in.  
 Glazing Angle: 90.0 °

**Glazing Construction (Double Glazed Insulating Unit)**

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**Exterior Lite Properties (Monolithic 3/16 in.)**

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*Single Glass Ply Properties*

RCSS (Heat Treatment): 0.00 psi (Annealed)  
 Min Thickness: 0.180 in.  
 Surface Treatment: None  
 Surface Parameters: 7.00 [1.36e-29 in<sup>12</sup>/lbf<sup>7</sup>] (ASTM)

**Airspace Properties**

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Thickness: 0.625 in.  
 Sealant Width: 0.236 in.  
 Elevation: 0.00 ft  
 Initial Pressure: 14.70 psi  
 Initial Temperature: 70.0 °F

**Interior Lite Properties (Monolithic 3/16 in.)**

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*Single Glass Ply Properties*

RCSS (Heat Treatment): 0.00 psi (Annealed)  
 Min Thickness: 0.180 in.  
 Surface Treatment: None  
 Surface Parameters: 7.00 [1.36e-29 in<sup>12</sup>/lbf<sup>7</sup>] (ASTM)

**Load Combinations**

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**Load Combination 1 - 70.0 psf (3.00 sec)**

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Description	Load	Duration	Factor	Total
Short Duration	70.0 psf	3.00 sec	1.00	70.0 psf

**Scenarios**

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**Scenario 1**

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Load Combination 1 acting on Exterior Lite 70.0 psf (3.00 sec)  
 Elevation (Atmospheric Pressure): 0.00 ft (14.6 psi)  
 Air Space 1 Temperature: 70.0 °F

## Details

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Selected standard:     ASTM E1300 Extended Basic

## Glazing Construction (Double Glazed Insulating Unit)

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### Exterior Lite Properties (3/16 in. Monolithic)

Construction:     3/16 in. (AN)

### Airspace Properties

Thickness:     0.625 in.

### Interior Lite Properties (3/16 in. Monolithic)

Construction:     3/16 in. (AN)

## Load Resistance

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### Short Duration (3 Sec)

<u>Description</u>	<u>NFL</u>	<u>GTF</u>	<u>LSF</u>	<u>LR</u>
Exterior Lite	44.0 psf	0.900	1/0.500	79.1 psf
Interior Lite	44.0 psf	0.900	1/0.500	79.1 psf

## Comparisons

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### Scenario 1

70.0 psf 3.00 sec <= 79.1 psf	OK
Approximate center of glass deflection	
Exterior Lite	0.56 in.
Interior Lite	0.56 in.

## Notes

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Load resistance values are computed in accordance with ASTM E1300-16 Section 6.2 and are based on non-factored load values calculated in a manner consistent with those presented in ASTM E1300-16.





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### Revision Log

<u>Rev. #</u>	<u>Date</u>	<u>Page(s)</u>	<u>Revision(s)</u>
0	09/28/23	N/A	Original report issue